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# Vigas de 2 materiales

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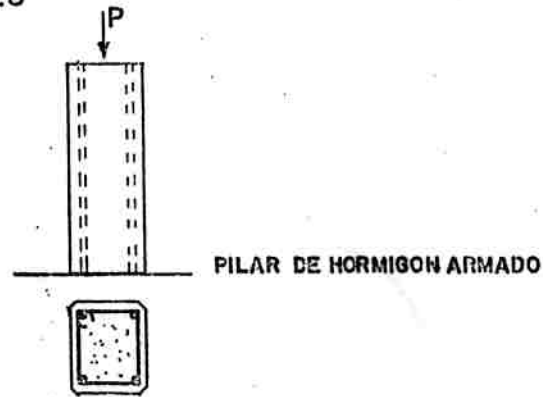
## Clase 17

Método general, Método de la Sección Transformada, Tensiones de Corte, Flexión Compuesta, Desplazamientos.

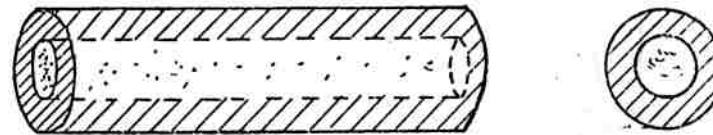


# BARRAS DE DOS MATERIALES

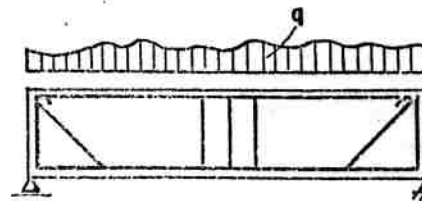
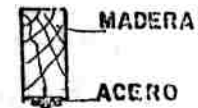
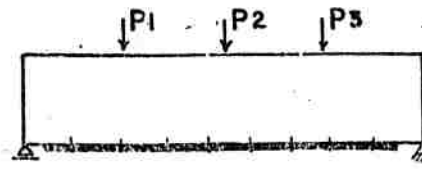
CARGAS AXIALES



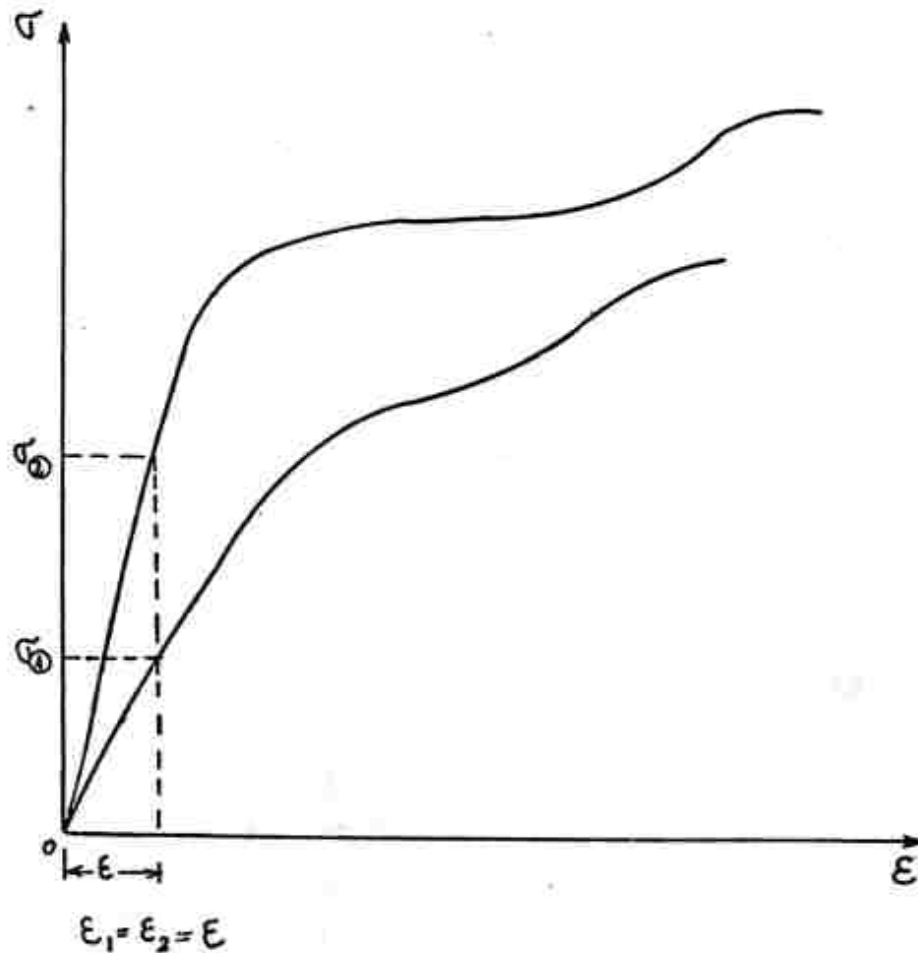
TORSION



FLEXION



**En vigas de dos materiales, en aquellos puntos en donde se le obligan a ambos materiales a trabajar juntos, a igual deformación corresponde tensiones diferentes**



## CASO1: CARGAS PARALELAS AL EJE

$$\int_{(1)} \sigma_1 dA + \int_{(2)} \sigma_2 dA = P \quad (1)$$

$$\int_{(1)} \sigma_1 y dA + \int_{(2)} \sigma_2 y dA = 0 \quad (2)$$

$$E_1 = E_2 = E = \frac{\sigma_1}{\epsilon} = \frac{\sigma_2}{\epsilon} \quad (3)$$

$$e_1 + e_2 = \gamma_{G1} + \gamma_{G2} \quad (4)$$

DE (1)  $\sigma_1 A_1 + \sigma_2 A_2 = P$   
 DE (3)  $\sigma_2 = \frac{E_2}{E_1} \sigma_1$

$$\sigma_1 = \frac{PE_1}{A_1 E_1 + A_2 E_2}$$

$$\sigma_2 = \frac{PE_2}{A_1 E_1 + A_2 E_2}$$

DE (2)  $\sigma_1 H_{EST1} = \sigma_2 H_{EST2}$   
 DE (4)  $e_1 + e_2 = \gamma_{G1} + \gamma_{G2}$

$$e_1 = \frac{(\gamma_{G1} + \gamma_{G2}) E_2 A_2}{A_1 E_1 + A_2 E_2}$$

$$e_2 = \frac{(\gamma_{G1} + \gamma_{G2}) A_1 E_1}{A_1 E_1 + A_2 E_2}$$

$$\sigma_1 = E_1 \epsilon$$

$$\sigma_2 = E_2 \epsilon$$

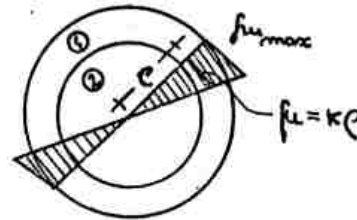
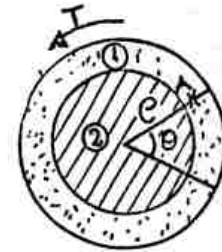
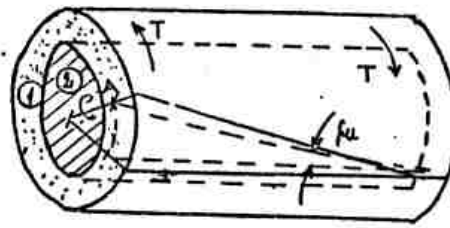


DIAGRAMA DE  $\tau_u$

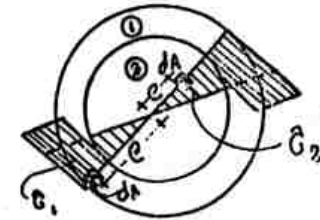


DIAGRAMA DE  $\tau$

## CASO 2: TORSION

$$\int_1 \rho \tau_1 dA + \int_2 \rho \tau_2 dA = T \quad \textcircled{1}$$

$$\tau_u = \kappa \rho$$

$$\tau_1 = \tau_u = \kappa \rho_1$$

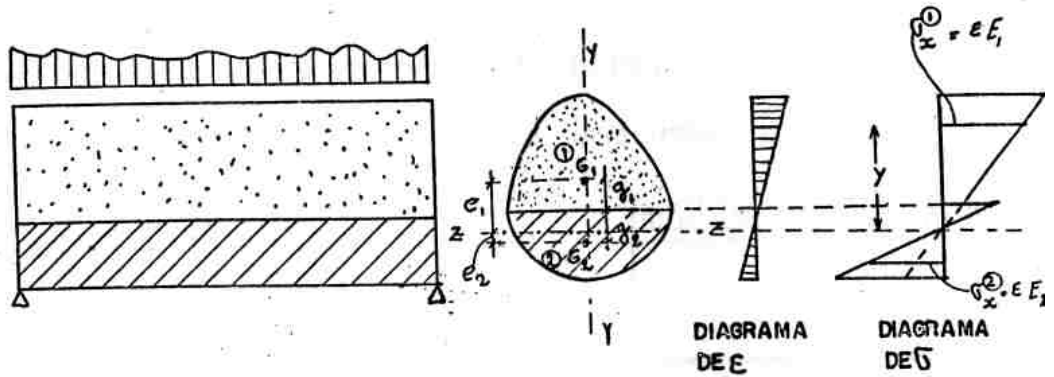
$$\tau_2 = \tau_u = \kappa \rho_2 \quad \textcircled{2}$$

$$\kappa = \frac{T}{G_1 I_{p1} + G_2 I_{p2}}$$

$$\tau_1 = \frac{T G_1 \rho_1}{G_1 I_{p1} + G_2 I_{p2}}$$

$$\tau_2 = \frac{T G_2 \rho_2}{G_1 I_{p1} + G_2 I_{p2}}$$

# CASO3: FLEXIÓN



$$\int \sigma_x^{(1)} dA + \int \sigma_x^{(2)} dA = 0 \quad (1)$$

$$\int \sigma_x^{(1)} y dA + \int \sigma_x^{(2)} y dA = M \quad (2)$$

$$\epsilon = \frac{y}{\rho}$$

$$\sigma_x^{(1)} = \epsilon E_1 = \frac{y E_1}{\rho} \quad (3)$$

$$\sigma_x^{(2)} = \epsilon E_2 = \frac{y E_2}{\rho} \quad (3)$$

$$e_1 + e_2 = g_1 + g_2 \quad (4)$$

DE (2) y (3)

$$\frac{E_1}{\rho} \int y^2 dA + \frac{E_2}{\rho} \int y^2 dA = M$$

$$\frac{1}{\rho} = \frac{M}{E_1 I_1 + E_2 I_2}$$

$$\sigma_x^{(1)} = \frac{M E_1 y}{E_1 I_1 + E_2 I_2}$$

$$\sigma_x^{(2)} = \frac{M E_2 y}{E_1 I_1 + E_2 I_2}$$

DE (1) y (3)

$$E_1 \int y dA = E_2 \int y dA$$

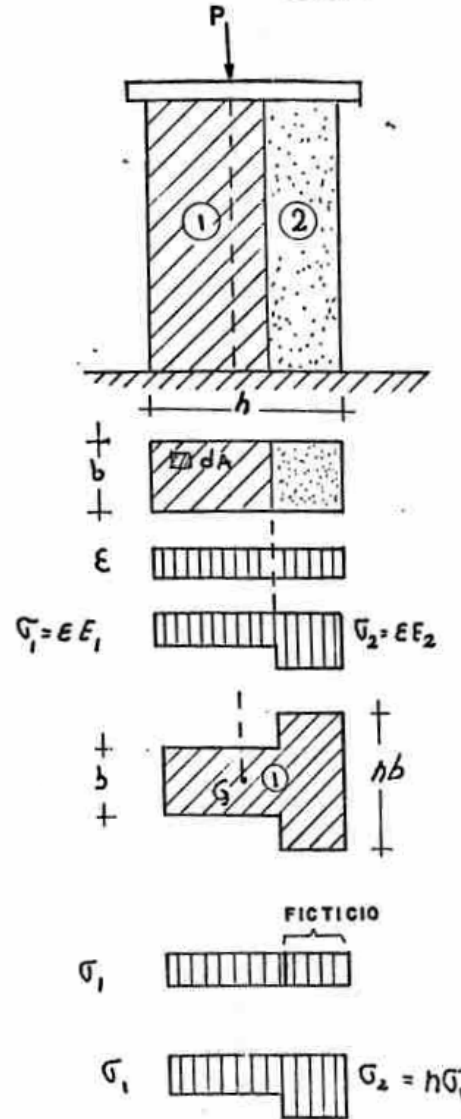
$$E_1 A_1 e_1 = E_2 A_2 e_2$$

y con (4)

$$e_1 = \frac{(g_1 + g_2) E_2 A_2}{E_1 A_1 + E_2 A_2}$$

$$e_2 = \frac{(g_1 + g_2) A_1 E_1}{E_1 A_1 + E_2 A_2}$$

CARGAS PARALELAS AL EJE



$$\sigma_1 = \frac{P E_1}{A_1 E_1 + A_2 E_2} = \frac{P}{A_1 + n A_2} = \frac{P}{A_{TRANS.}}$$

$$\frac{E_2}{E_1} = n \quad \downarrow \quad \begin{aligned} A_2 &= b h_2 \\ n A_2 &= (n b) h_2 \end{aligned}$$

$$dF_1 = \sigma_1 dA \quad dF_2 = \sigma_2 dA$$

$$dF_1 = E \epsilon_1 dA \quad dF_2 = E \epsilon_2 dA$$

$$\frac{E_2}{E_1} = n = \frac{\sigma_2}{\sigma_1} \quad \downarrow$$

$$dF_2 = E n \epsilon_1 dA$$

$$dF_2 = E \epsilon_1 (n dA)$$

↓

AREA EQUIVALENTE A ①

$$\int_{①} \sigma_1 y dA = \int_{②} \sigma_2 y dA$$

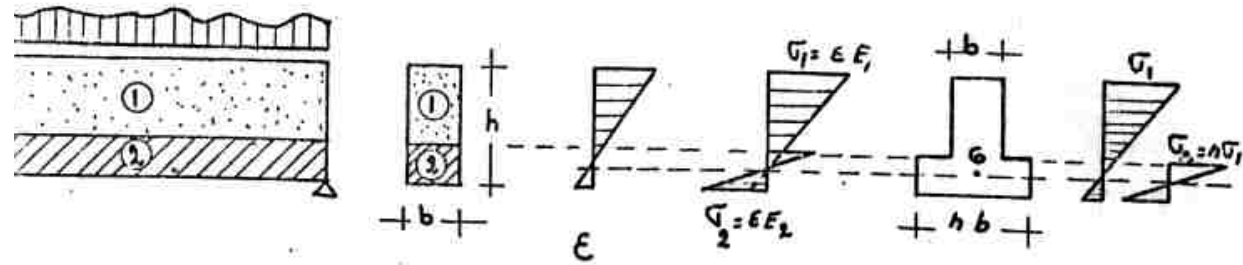
$$\frac{\sigma_2}{\sigma_1} = \frac{E_2}{E_1} = n$$

$$\int_{①} y dA = \int_{②} y (n dA)$$

**METODO DE LA SECCIÓN TRANSFORMADA**

SE DEBE ALTERAR EL ANCHO PORQUE LA DISTANCIA "Y" NO DEBE ALTERARSE

# FLEXIÓN: METODO DE LA SECCIÓN TRANSFORMADA



$$\sigma_1 = \frac{ME \cdot y}{\frac{I_1 E_1 + I_2 E_2}{E_1}} = \frac{MY}{\frac{I_1}{E_1} + n I_2} = \frac{MY}{I_{TRANS}}$$

$$\frac{E_2}{E_1} = n = \frac{\sigma_2}{\sigma_1} \quad I_2 = \frac{bh^3}{12} \quad n I_2 = (nb) \frac{h^3}{12}$$

$$dF_1 = \sigma_1 dA = \frac{E_1 y dA}{\rho}$$

$$dF_2 = \sigma_2 dA = \frac{E_2 y dA}{\rho} = \frac{E_1 y (n dA)}{\rho}$$

AREA EQUIVALENTE AL MAT. 1

$$E_1 \int_1 y dA = E_2 \int_2 y dA$$

$$\int_1 y dA = \int_2 y (n dA) \rightarrow$$

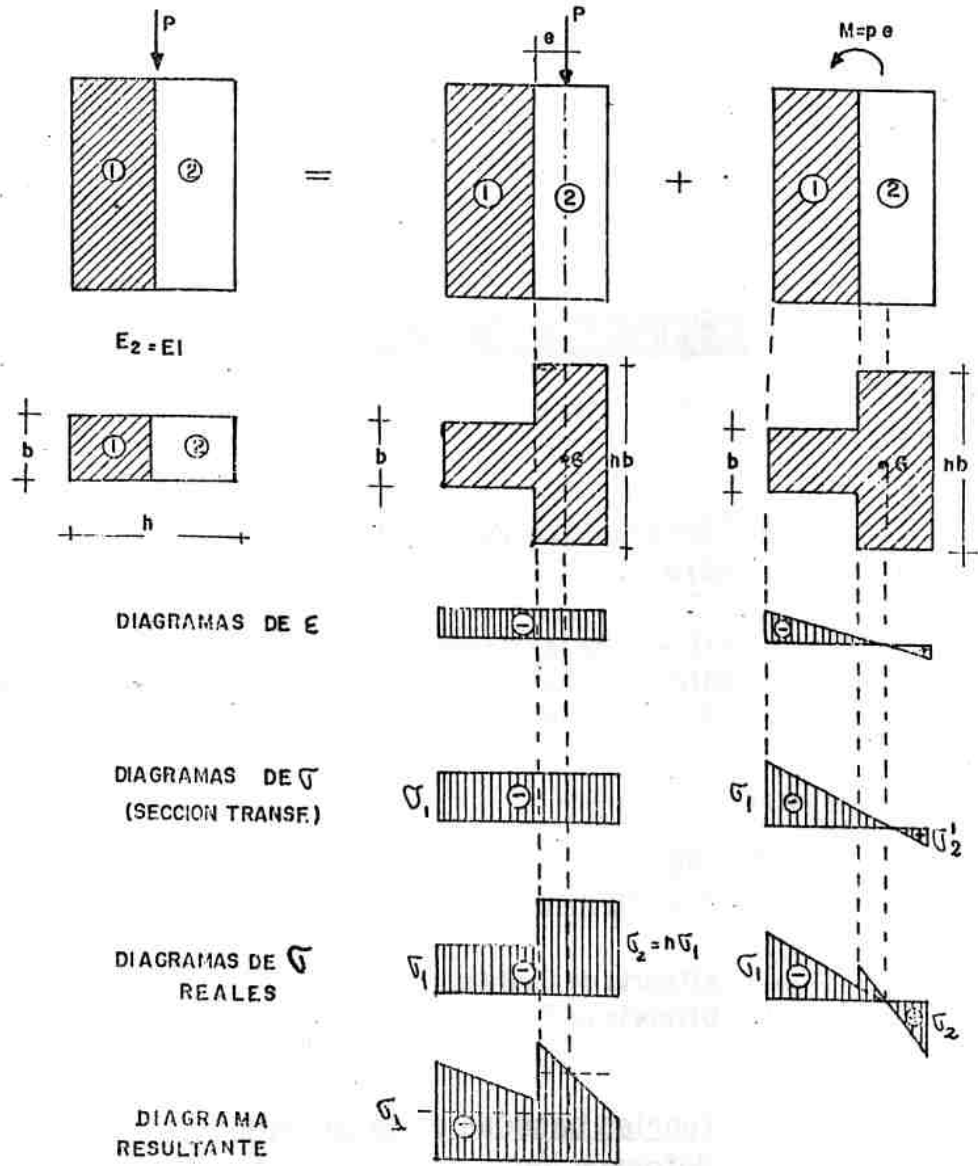
El Eje Neutro no cambiará si cada elemento del cuerpo 2 es multiplicado por un factor "n", siempre que no se altere la distancia "y" de cada uno de ellos al mismo eje. O sea: SOLO SE PUEDE VARIAR EL ANCHO

La resistencia de la Sección Transformada es la misma que la original

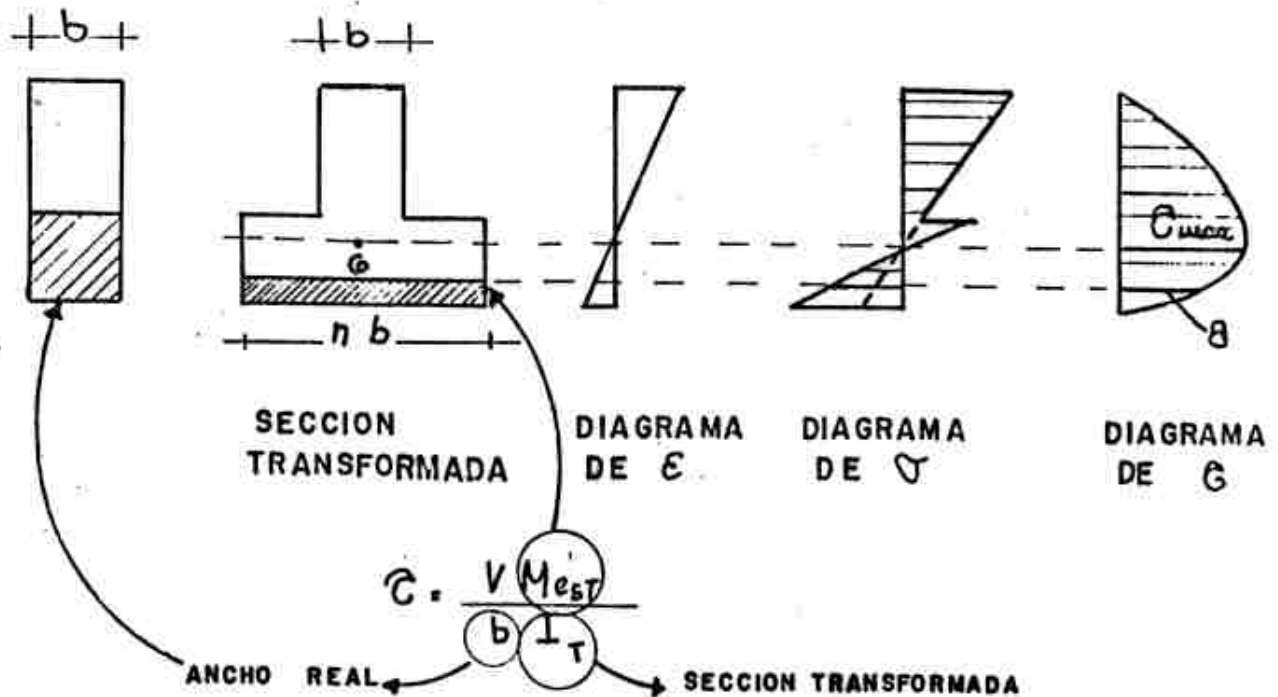
$$M = \int_A \sigma \cdot y \cdot dA = \int_1 \sigma_1 \cdot y \cdot dA + \int_2 \eta \cdot \sigma_1 \cdot y \cdot dA = \frac{1}{\rho} \cdot (E_1 \cdot I_1 + E_2 \cdot I_2)$$



# FLEXION COMPUESTA



# TENSION DE CORTE EN LA FLEXIÓN



## DEFORMACIONES

Las deformaciones y los desplazamientos se pueden determinar usando la Sección Transformada. Esta sección representa la sección recta de un elemento hecho de un material homogéneo que se deforma de la misma manera que el elemento compuesto

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# Próxima Clase: Estado plano de Tensiones

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Fin